

ADDITIONAL INFORMATION AND COMPUTATIONS

USGS RURAL REGRESSION EQUATIONS FROM SCIENTIFIC INVESTIGATIONS REPORT 2009-5158 DA = 2.74 SQ. MI.IMPERVIOUS AREA % = <10% (FUTURE IMPERVIOUS FROM GUILFORD COUNTY ZONING DATA) REGION 1 – PIEDMONT RURAL RIDGE and VALLEY-PIEDMONT (SIR 2009-5158) SAY $Q_{10} = 398 (2.74) = 741 cfs$ 740 cfs $Q_{25} = 537 (2.74) = 989 \text{ cfs}$ 990 cfs $Q_{50} = 661 (2.74) = 1,210 \text{ cfs}$ 1,200 cfs $Q_{100} = 776 (2.74) = 1,412 cfs$ 1,400 cfs $Q_{500} = 1,072 (2.74) = 1,929 \text{ cfs}$ 1,900 cfs

NOTES:

- 1. NO UPSTREAM OR DOWNSTREAM STRUCTURES THAT WERE IN PLACE AT THE TIME THIS PROJECT WAS DESIGNED WILL BE ADVERSELY AFFECTED BY THE PROPOSED CULVERT.
- 2. STREAM BED MATERIAL IS SANDY CLAY.
- 3. JEFFREY BROWN, THE NCDOT DIVISION 7 TRANSPORTATION SUPERVISOR, STATED THAT HE HAS NOT SEEN THE WATER OVERTOP THE ROADWAY AT THE PROJECT LOCATION.

4. NATIVE MATERIAL BETWEEN SILLS/BAFFLES IN THE CULVERT SHALL PROVIDE A CONTINUOUS LOW

FLOW CHANNEL. NATIVE MATERIAL CONSISTS OF MATERIAL THAT IS EXCAVATED FROM THE THE STREAM OR FLOODPLAIN AT THE PROJECT SITE DURING CONSTRUCTION. ONLY MATERIAL THAT IS EXCAVATED FROM THE STREAM BED MAY BE USED TO LINE THE LOW FLOW CULVERT BARREL. RIP RAP MAY BE USED TO SUPPLEMENT THE NATIVE MATERIAL IN THE HIGH FLOW CULVERT BARREL(S). IF RIP RAP IS USED TO LINE THE HIGH FLOW CULVERT BARREL(S), NATIVE MATERIAL SHOULD BE PLACED ON TOP TO FILL VOIDS AND PROVIDE A FLAT SURFACE FOR ANIMAL PASSAGE. NATIVE MATERIAL IS SUBJECT TO APPROVAL BY THE ENGINEER AND MAY BE SUBJECT TO PERMIT CONDITIONS.

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SITE DATA

USGS STREAM STATS/USGS QUAD MAI 2.74 SQ. MI REGION 1-PIEDMONT; RURAL CAPE FEAR River Basin Character Stream Classification (Such as Trout, High Quality Water, etc.)WS-IV; NSW 1 SPAN, 1@33'-4" TIMBER DECK ON I-BEAMS, 14' BED TO CROWN Data on Existing Structure Total Waterway Opening 306.5 s.f. Waterway Opening Below 100yr. WS EL. 204.8 s.f. Debris Potential: Low ☐ Moderate ☒ High ☐ Data on Structures Up and Down Stream 0.7 MILES UPSTREAM: NON-NBIS STR. NO. E131 ON SR 2719 2 @ 7' DIAMETER CMP WITH CONCRETE HEADWALL. DOWNSTREAM: CONFLUENCE WITH HAW RIVER. Period of Records Gage Station No. NA Max. Discharge N/A c.f.s Date N/A Frequency N/A Historical Flood Information: SEE NOTE 3 IN ADDITIONAL INFORMATION JEFFREY BROWN (NCDOT DIV. 7 Period of Elev. NO OT ft. Est. Freq. yr. Source TRANSPORTATION SUPERVISOR) Knowledge N/A yrs. Date 12/2018 Elev. 628.2 ft. Est. Freq. <10 yr. Source RECENT HIGH WATER Period of (SURVEYED ON 1/18/2019) Knowledge N/A yrs. Allowable HW Elev. 630.5 (25-YR EXISTING @ RS 634) ft. Normal Water Surface Elev. 623.4 ft. Manning's n: Left O.B. 0.14 Channel 0.05 Right O.B. 0.14 Obtained From FIELD SURVEY GUILFORD CO. FIS – NOV. 17, 2017, NO FEMA STUDY
Flood Study /Status BACKWATER OF HAW RIVER, ZONE AE, PANEL #8930 Floodway Established? NO Flood Study 100 yr. Discharge N/A c.f.s.; WS Elev.: Floodway N/A ft. Without Floodway DESIGN DATA

Hydrological Method USGS RURAL REGRESSION EQUATIONS - SCIENTIFIC INVESTIGATIONS REPORT 2009-5158 HEC-RAS 5.0.6 FILENAME: 400183 UTtoHawRiver SR2710.prj Hydraulic Design Method

Design Tailwater : $Q_{10} = 6.4$ ft.; $Q_{25} = 7.3$ ft.; $Q_{50} = 7.9$ ft.; $Q_{100} = 8.5$ ft.; $Q_{500} = 9.7$ ft.

| | | | Inlet | Control | | | Remarks | | | | | | |
|--|---------------|----|-------|---------------|----|------------------|----------------|---|-----|---------------|---------|--|--|
| Size & Type | Q (c.f.s.) | Кe | HW/D | H.W. (ft.) | dc | $\frac{dc+D}{2}$ | h _o | Н | LSo | H.W. (ft.) | Remarks | | |
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| CEE HEC DAC MACDEL | | | | | | | | | | | | | |
| SEE HEC-RAS MODEL | | | | | | | | | | | | | |
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| Floodway Revision Required? NO Total Proposed Waterway Opening 204 | | | | | | | | | | | | | |

| Required | Outlet Protection | CLASS II | | | | | |
|----------|-------------------|-----------|------------------|-------|-------|----|--|
| | INFORMATION | OT NC | BE SHOWN | ON PL | ANS | | |
| | | | River Station 63 | | | | |
| Design: | Discharge 990 c | .f.s. Fre | quency 25 | ōyr. | Elev. | 63 | |

Outlet Velocity (V_{10}) 5.1 f.p.s. Natural Channel Velocity (V_{10})

Base Flood: Discharge 1,400 c.f.s. Frequency 100 yr. Elev. Overtopping: Discharge 2,900 c.f.s. Frequency 500+ yr. Elev. *OT OCCURS AT SAG _L_ STA. 14+91.6 RT *639.1

CULVERT SURVEY & HYDRAULIC DESIGN REPORT

N. C. DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS HYDRAULICS UNIT RALEIGH, N. C.

♀ I.D. No. SF–400183 Project No. 17BP.7.R.134 Proj. Station 14 + 77.0 -LUT TO HAW RIVER GUILFORD Stru. No. 0183 On Highway SK Z/10
(RUNNING CREEK RD) SR 2719 SR 2711 3K 2/19 and (HIGH ROCK RD) (TROXLER MILL RD) 2 @ 12' X 10' RCBC w/TOP EDGE BEVEL AND WINGWALLS, Recommended Structure SILLS AT INLET & OUTLET (1' HIGH IN WEST BARREL, 2' HIGH IN EAST BARREL) Recommended Width of Roadway 40'-0" SHOULDER PT. TO SHOULDER PT. Skew 11.6' DOWNSTREAM Recommended Location is (Up, At, (Down)) Stream from Existing Crossing. Latitude . 36.23330 -79.56108 Statewide Tier \square Regional Tier 🗌 Sub-Regional Tier X Bench Mark is BM1, -L- STA. 14+35.64, 112.00' RT, 60D NAIL IN 12" POPLAR Northing: 904232 Easting: 1834425 Elev. 634.30 ft. Datum: NAVD 88 Temporary Crossing NOT REQUIRED, OFFSITE DETOUR PROVIDED



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Erik Seiler

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f.p.s.





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